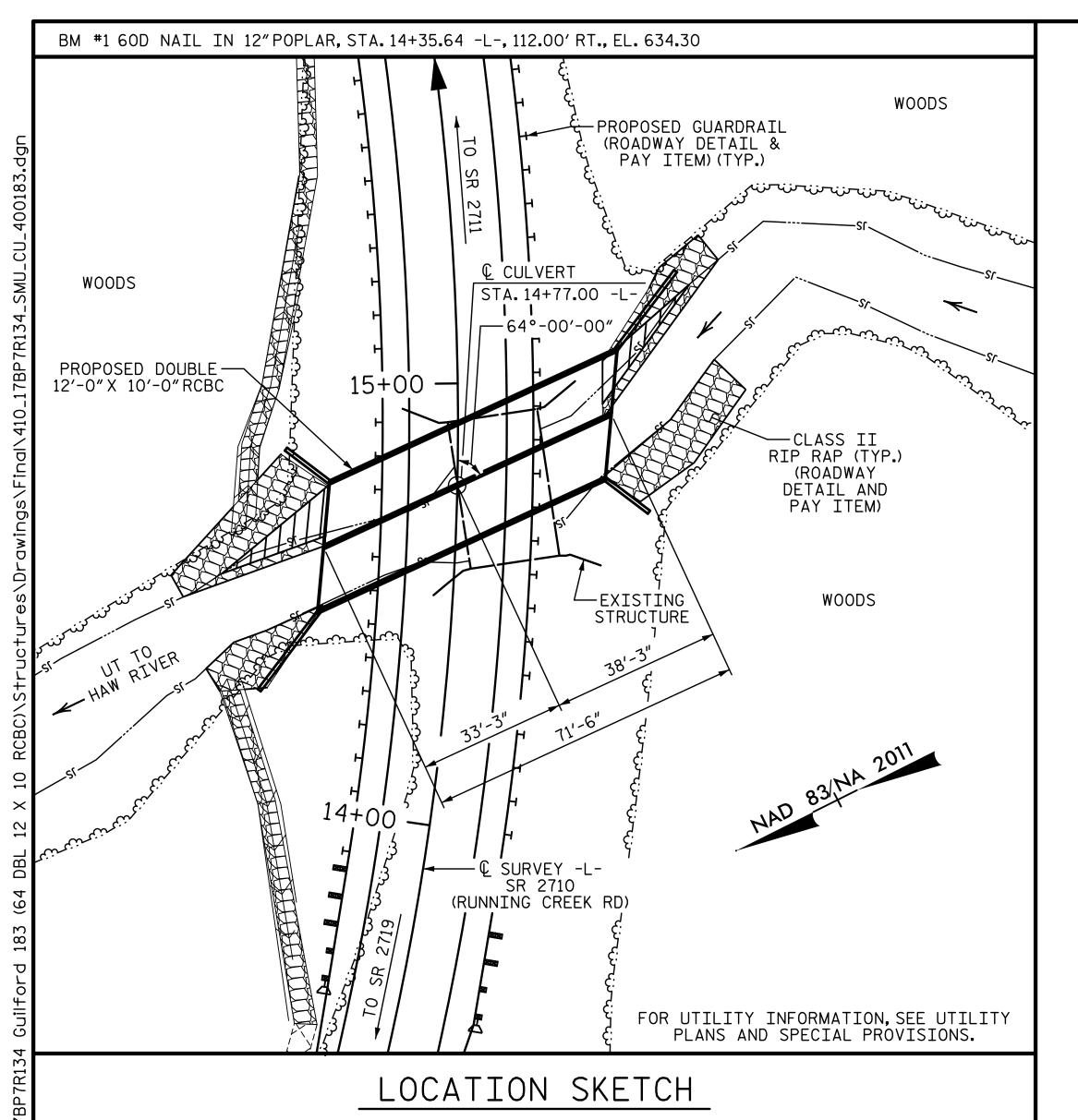
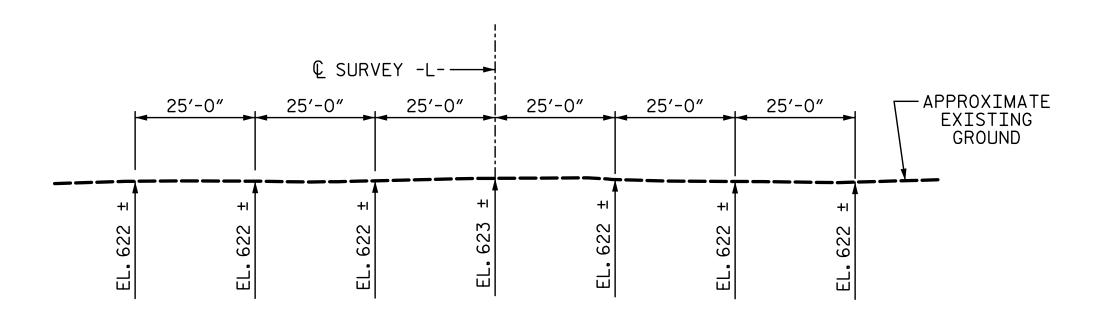
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PROFILE ALONG & CULVERT

HYDRAULIC DATA:

DESIGN DISCHARGE = 990 CFS = 25 YEAR FREQUENCY OF DESIGN FLOOD = 630.30 DESIGN HIGH WATER ELEVATION = 2.74 SQ. MI. DRAINAGE AREA BASE DISCHARGE (Q 100) = 1400 CFS BASE HIGH WATER ELEVATION = 631.80

OVERTOPPING FLOOD DATA:

OVERTOPPING DISCHARGE = 2900 CFS FREQUENCY OF OVERTOPPING FLOOD = 500+ YEAR = 639**.**10 \*\* OVERTOPPING FLOOD ELEVATION \*\*OVERTOPPING OCCURS AT

SAG AT STA. 14+91.60 -L- AT ROADWAY (HIGH) RIGHT SIDE

### GRADE DATA:

GRADE POINT EL. @ STA. 14+77.00 -L- = EL. 638.20 BED EL. @ STA. 14+77.00 -L- = EL. 621.36 ROADWAY SLOPE 2:1

### TOTAL STRUCTURE QUANTITIES LUMP SUM REMOVAL OF EXISTING STRUCTURE LUMP SUM ASBESTOS ASSESSMENT LUMP SUM BOX CULVERT EXCAVATION. 151 TONS FOUNDATION CONDITIONING MATERIAL CLASS A CONCRETE 2.916 CY/FT 208.5 c.Y. BARREL @ 2.8 c.Y HEADWALLS 2.7 c.y 67.5 c.Y. WING ETC. 281.5 c.y. REINFORCING STEEL 32,021 LBS. BARREL 7,552 LBS. WINGS ETC. 39,573 LBS.

### FOUNDATION NOTES:

EXCAVATE FOUNDATION A MINIMUM OF 1.0 FEET BELOW CULVERT BEARING ELEVATION. PLACE 1.0 FEET OF CLASS VI FOUNDATION CONDITIONING MATERIAL IN ACCORDANCE WITH SECTION 414 OF THE STANDARD SPECIFICATION.

OVEREXCAVATE LOOSE/SOFT MATERIAL IF PRESENT TO SUITABLE BEARING MATERIALS AND REPLACE WITH ADDITIONAL CLASS VI FOUNDATION CONDITIONING MATERIAL.

### NOTES:

ASSUMED LIVE LOAD ------HL-93 OR ALTERNATE LOADING.

DESIGN FILL------ 5'-6"(MIN.) AND 7'-9"(MAX.)

FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

- CONCRETE IN CULVERT TO BE POURED IN THE FOLLOWING ORDER: 1. EASTERN BARREL WING FOOTINGS, FLOOR SLAB, AND WESTERN BARREL WING FOOTINGS TO CONSTRUCTION JOINT (STAGE I) INCLUDING 4"OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE WALLS AND EASTERN BARREL WINGS FULL HEIGHT AND 1'-6" OF WESTERN BARREL WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.
- 3. REMAINING PORTION OF WESTERN BARREL WING FOOTINGS INCLUDING 4"OF WINGS FOLLOWED BY FULL HEIGHT OF WESTERN BARREL WINGS (STAGE II).

THE ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

STEEL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITTED CONSTRUCTION JOINT AT THE CONTRACTOR'S OPTION, EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTS OF 1 SPAN @ 33'-4"WITH A CLEAR ROADWAY WIDTH OF 19'-3". THE SUPERSTRUCTURE CONSISTS OF TIMBER DECK WITH A 2"ASPHALT WEARING SURFACE ON STEEL I-BEAMS. THE SUBSTRUCTURE CONSISTS OF TIMBER CAPS ON CONCRETE ENCASED TIMBER PILES WITH TIMBER BULKHEAD END BENTS. THE EXISTING STRUCTURE, WHICH IS LOCATED AT THE SITE OF THE PROPOSED STRUCTURE, SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT, SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED CULVERT, THE LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

- FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COST RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR REMOVAL OF EXISTING STRUCTURE AT STATION 14+77.00 -L-.

> PROJECT NO. <u>17BP.7.R.134</u> GUILFORD COUNTY 14+77.00 -L-STATION:

SHEET 1 OF 6 REPLACES BRIDGE #183

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3/4/2024 | 11:53 AM

DOUBLE 12 FT. X 10 FT. CONCETE BOX CULVERT

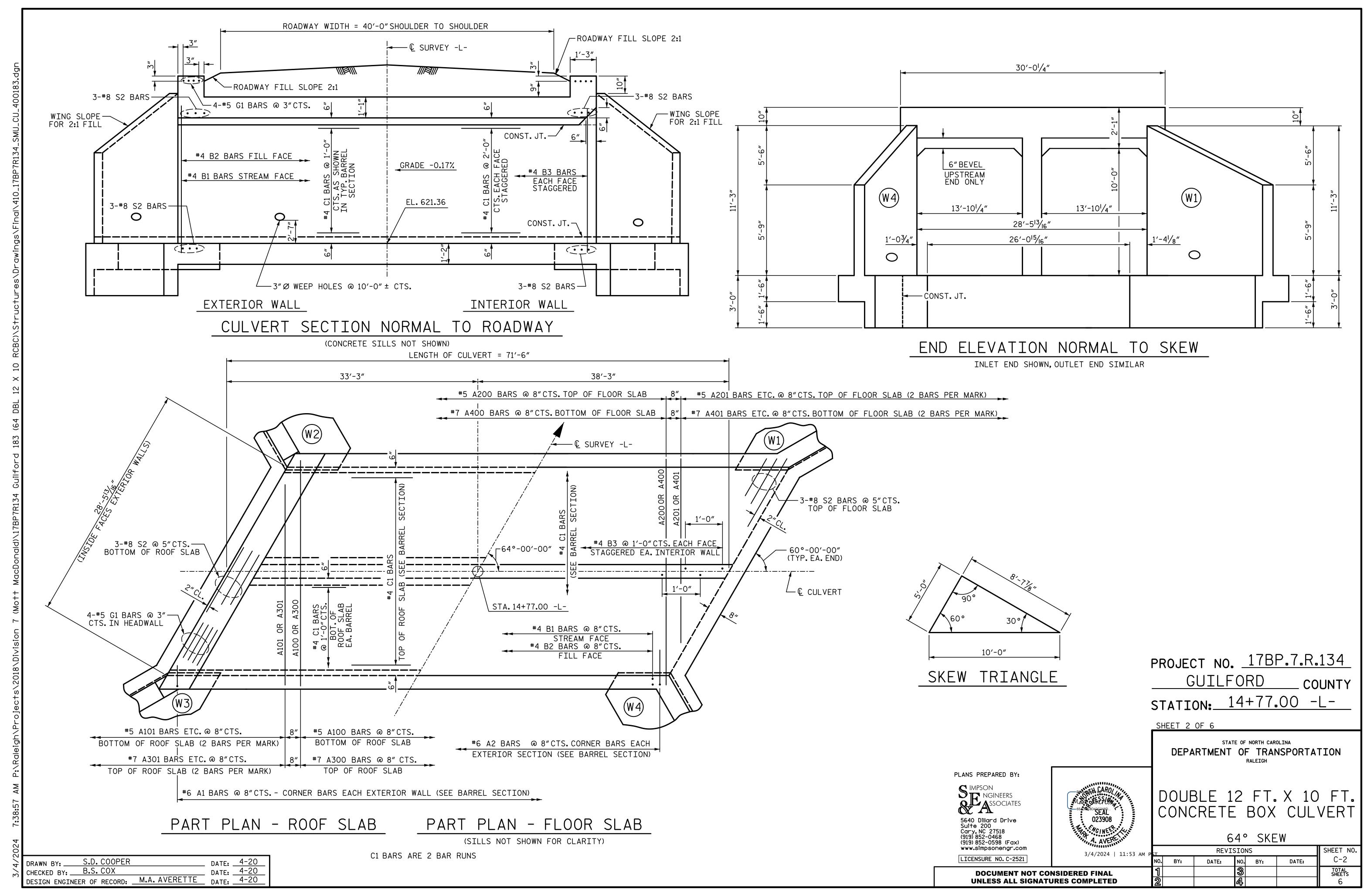
64° SKEW

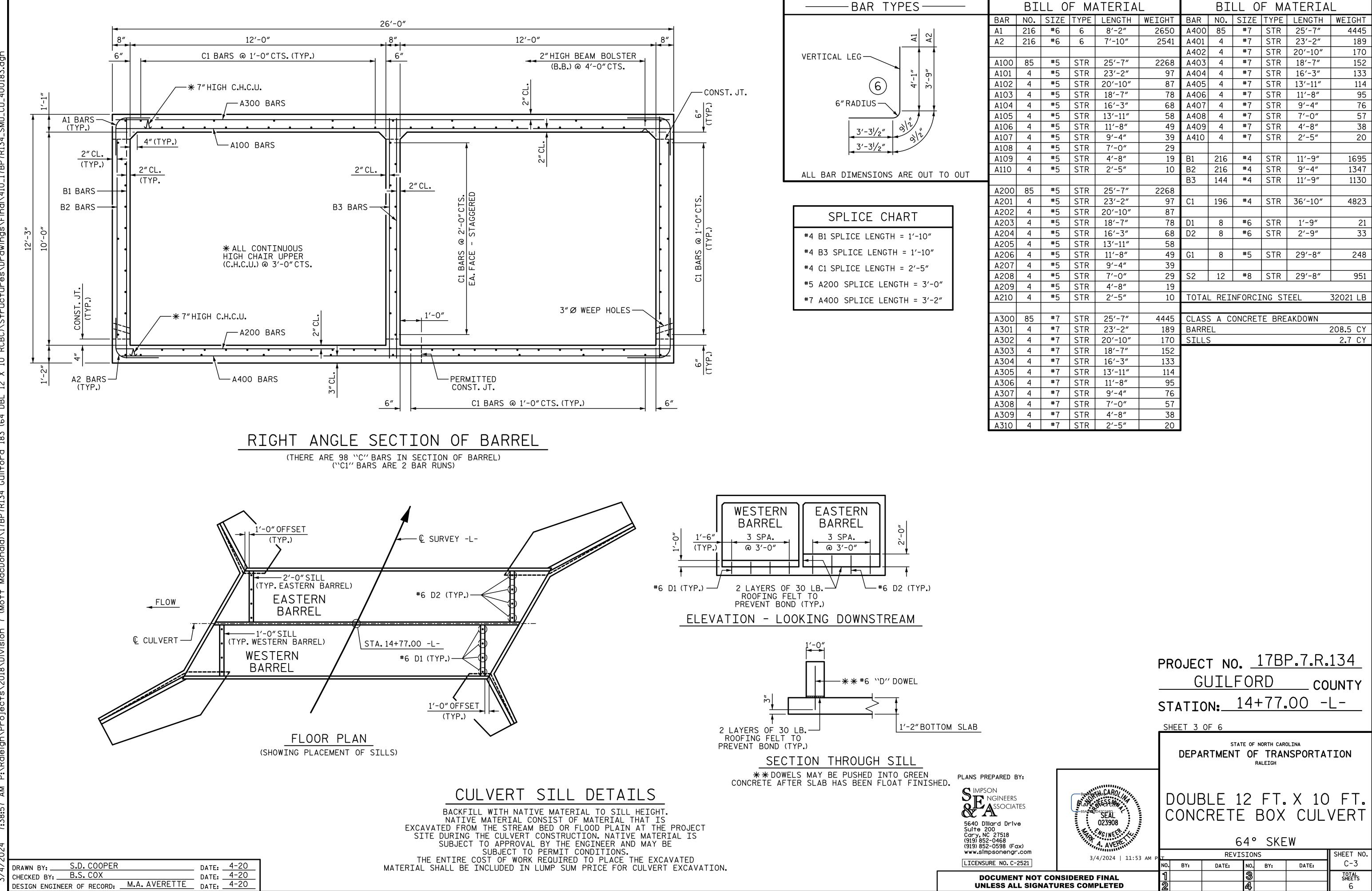
**REVISIONS** SHEET NO. C-1 NO. BY: BY: DATE: DATE: TOTAL SHEETS

PLANS PREPARED BY: NGINEERS ASSOCIATES 5640 Dillard Drive Suite 200 Cary, NC 27518 (919) 852-0468 (919) 852-0598 (Fax) www.simpsonengr.com LICENSURE NO. C-2521

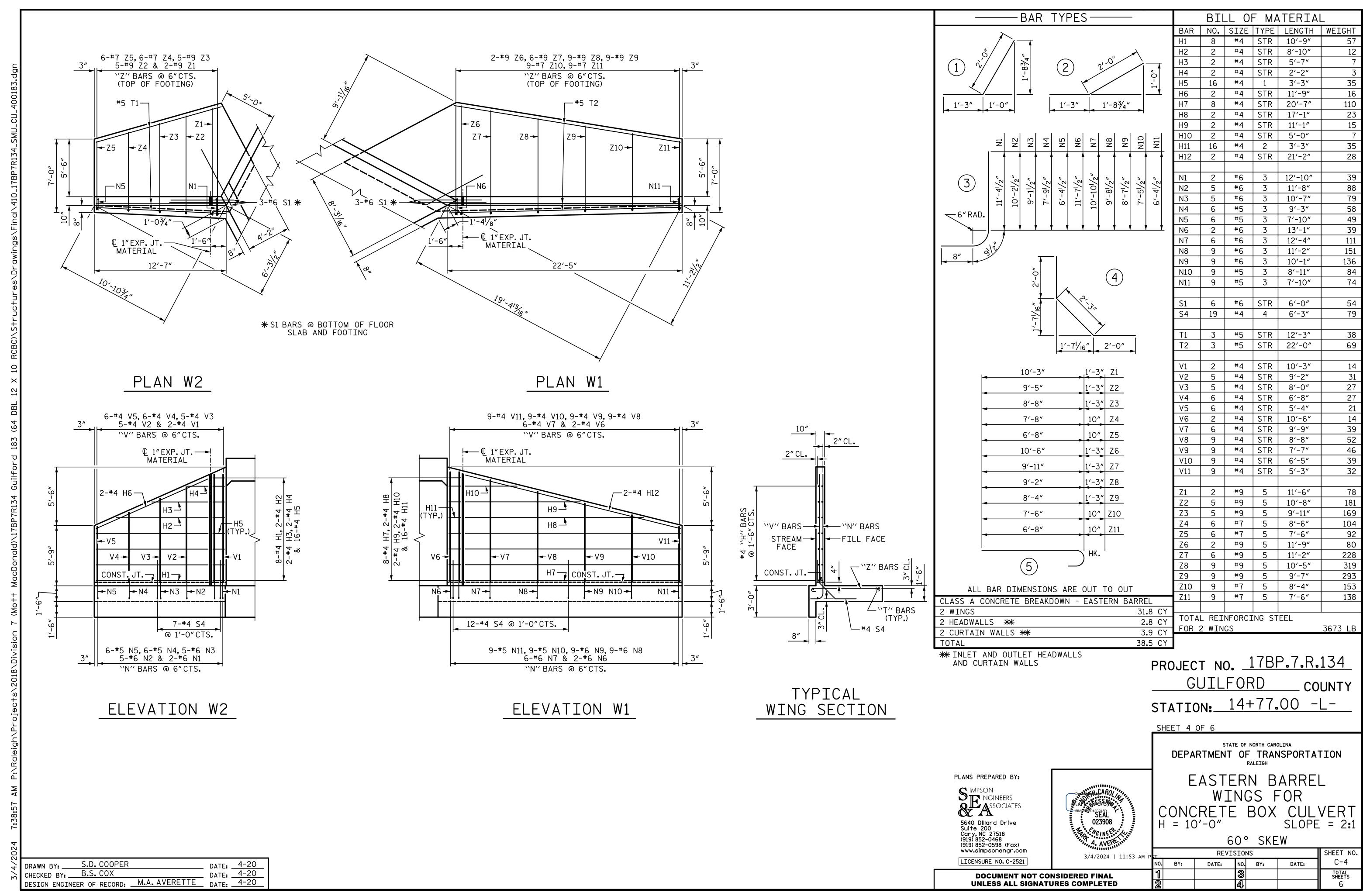
**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

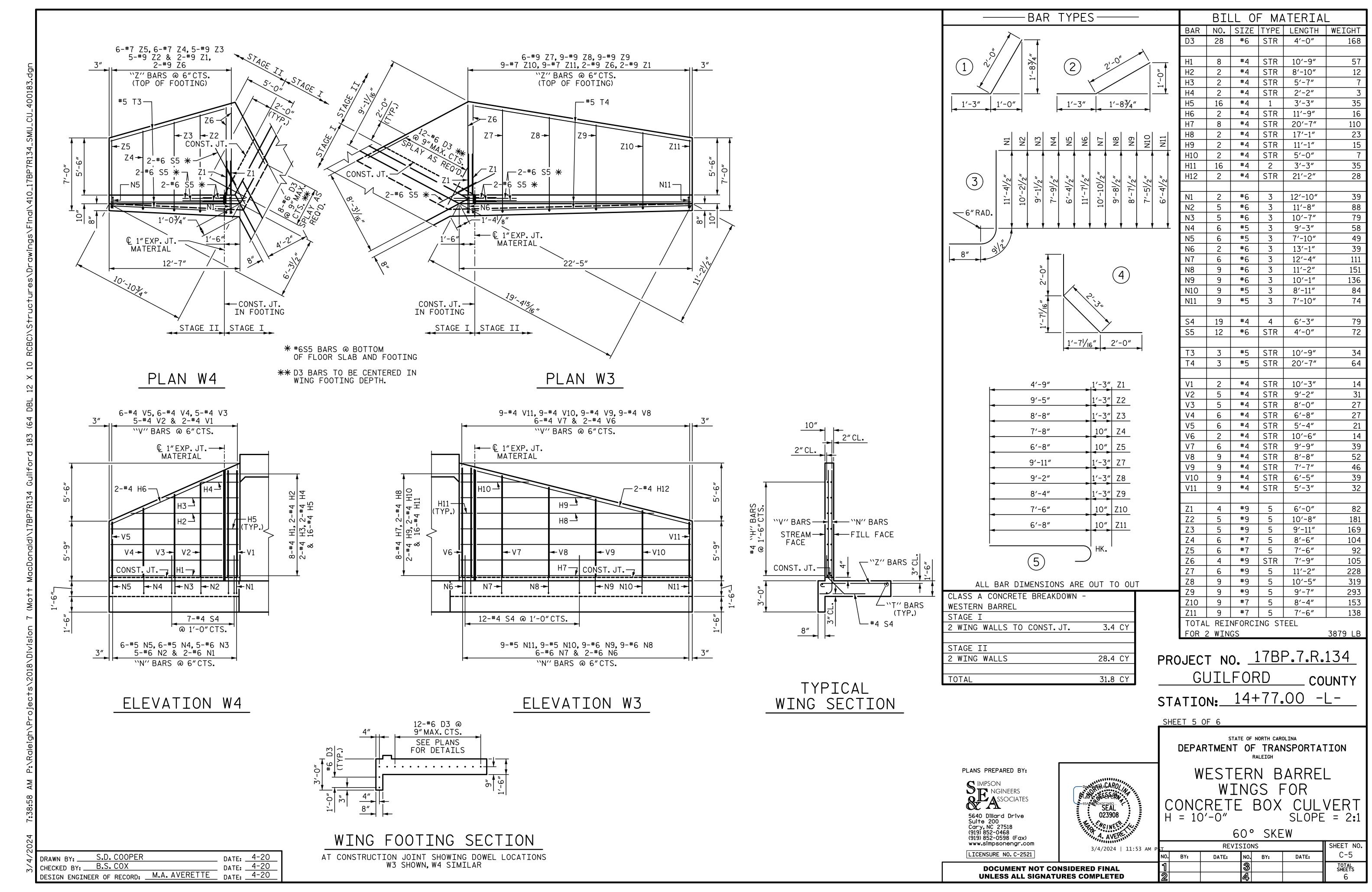
S.D. COOPER CHECKED BY: B.S. COX DATE: 4-20 DATE: 4-20 DESIGN ENGINEER OF RECORD: M.A. AVERETTE





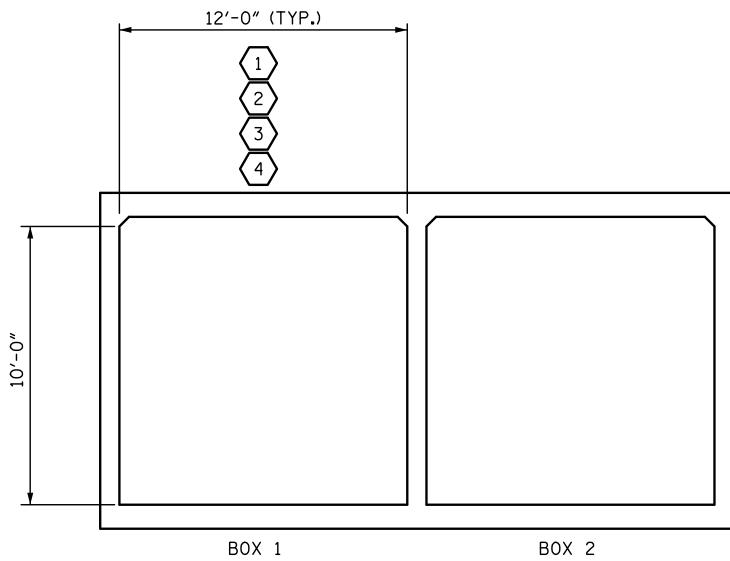
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# LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

	STRENGTH I LIMIT STATE															
LOAD TYPE								MOMENT				SHEAR				
		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
DESIGN LOAD		HL-93 (INVENTORY)	N/A	1	1.28		1.75	1.28	1	TOP SLAB - MID	6.00	1.86	1	BOT SLAB - RT END	10.8	
		HL-93 (OPERATING)	N/A		1.66		1 <b>.</b> 35	1.66	1	TOP SLAB - MID	6.00	2.41	1	BOT SLAB - RT END	10.8	
RATING		HS-20 (INVENTORY)	36.000	2	1.34	48.2	1.75	1.34	1	TOP SLAB - MID	6.00	1.91	1	BOT SLAB - RT END	10.8	
		HS-20 (OPERATING)	36.000		1.74	62.5	1 <b>.</b> 35	1.74	1	TOP SLAB - MID	6.00	2.48	1	BOT SLAB - RT END	10.8	
	LE VEHICLE (SV)	SNSH	13.500		2.13	28.8	1.40	2.13	1	EXT WALL - MID	5.00	2.19	1	EXT WALL - BOT END	0.7	
		SNGARBS2	20.000		2.02	40.4	1.40	2.02	1	TOP SLAB - MID	6.00	2.19	1	EXT WALL - BOT END	0.7	
		SNAGRIS2	22.000		2.13	46.9	1.40	2.13	1	EXT WALL - MID	5.00	2.19	1	EXT WALL - BOT END	0.7	
		SNCOTTS3	27.250		1.28	34.9	1.40	1.28	1	TOP SLAB - MID	6.00	1.86	1	TOP SLAB - RT END	10.9	
		SNAGGRS4	34.925	3	1.20	41.9	1.40	1.20	1	TOP SLAB - MID	6.00	1.69	1	TOP SLAB - RT END	10.9	
	SINGL	SNS5A	35.550		1.28	45.5	1.40	1.28	1	TOP SLAB - MID	6.00	1.80	1	TOP SLAB - RT END	10.9	
	0)	SNS6A	39.950		1.22	48.7	1.40	1.22	1	TOP SLAB - MID	6.00	1.76	1	BOT SLAB - RT END	10.8	
LEGAL		SNS7B	42.000		1.24	52.1	1.40	1.24	1	TOP SLAB - MID	6.00	1.68	1	TOP SLAB - RT END	10.9	
LOAD	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		1.82	60.1	1.40	1.82	1	TOP SLAB - MID	6.00	2.03	1	BOT SLAB - RT END	10.8	
		TNT4A	33.075		1 <b>.</b> 53	50.6	1.40	1 <b>.</b> 53	1	TOP SLAB - MID	6.00	2.12	1	TOP SLAB - RT END	10.9	
		TNT6A	41.600		1.35	56.2	1.40	1.35	1	TOP SLAB - MID	6.00	1.79	1	TOP SLAB - RT END	10.9	
		TNT7A	42.000		1.48	62.2	1.40	1.48	1	TOP SLAB - MID	6.00	1.76	1	BOT SLAB - RT END	10.8	
		TNT7B	42.000		1.23	51.7	1.40	1.23	1	TOP SLAB - MID	6.00	1.73	1	TOP SLAB - RT END	10.9	
		TNAGRIT4	43.000		1 <b>.</b> 52	65.4	1.40	1.52	1	TOP SLAB - MID	6.00	1.73	1	BOT SLAB - RT END	10.8	
		TNAGT5A	45.000		1.58	71.1	1.40	1.58	1	TOP SLAB - MID	6.00	1.75	1	BOT SLAB - RT END	10.8	
		TNAGT5B	45.000		1.47	66.2	1.40	1.47	1	TOP SLAB - MID	6.00	1.54	1	BOT SLAB - RT END	10.8	
EMERGENCY VEHICLE (EV)		TNAGT5B	45.000		1.42	40.8	1.30	1.42	1	TOP SLAB - MID	6.00	2.00	1	TOP SLAB - RT END	10.9	
		TNAGT5B	45.000	4	1.03	44.3	1.30	1.03	1	TOP SLAB - MID	6.00	1.48	1	TOP SLAB - RT END	10.9	



# LRFR SUMMARY

(LOOKING DOWNSTREAM)

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR		
DC	1.25	0.90		
DW	1.50	0.65		
EV	1.30	0.90		
EH	1.35	0.90		
ES	1.35	0.90		
LS	1.75			
WA	1.00			

NOTE:

PLANS PREPARED BY:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING \*\*

3 EMERGENCY VEHICLE LOAD RATING \*\* \*\* SEE CHART FOR VEHICLE TYPE

> PROJECT NO. <u>17BP.7.R.134</u> GUILFORD \_\_\_ COUNTY STATION: 14+77.00 -L-

SHEET 6 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

(NON-INTERSTATE TRAFFIC)

5640 Dillard Drive Suite 200 Cary, NC 27518 (919) 852-0468 (919) 852-0598 (Fax) www.simpsonengr.com 3/4/2024 | 11:53 AM LICENSURE NO. C-2521 C-6 NO. BY: DATE: NO. BY: DATE: DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED** 

SIMPSON
NGINEERS
ASSOCIATES

CHECKED BY: B.S. COX DATE: 4-20
DESIGN ENGINEER OF RECORD: M.A. AVERETTE DATE: 4-20

## STANDARD NOTES

### DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W -	27,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50 -	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT:

### ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.
ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND

CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE
AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL
BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE
FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED, DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8"Ø SHEAR STUDS FOR THE 3/4"Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8"Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4"Ø STUDS BASED ON THE RATIO OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE".

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.